

Stormwater Management Report

SUBMIT TO:

Department of Environmental Protection and Sustainability

PROJECT:

Tidal Wave MDB230016.00

PROJECT LOCATION:

23178 Three Notch Road California, MD 20619 St. Mary's County

DEVELOPER:

PJ Land, LLC Contact: Mike McGrath 631-449-3791

mimcgrath@pjlandllc.com

Bohler

16701 Melford Blvd, Suite 310 Bowie, MD 20715 Contact: Matt Senenman msenenman@bohlereng.com

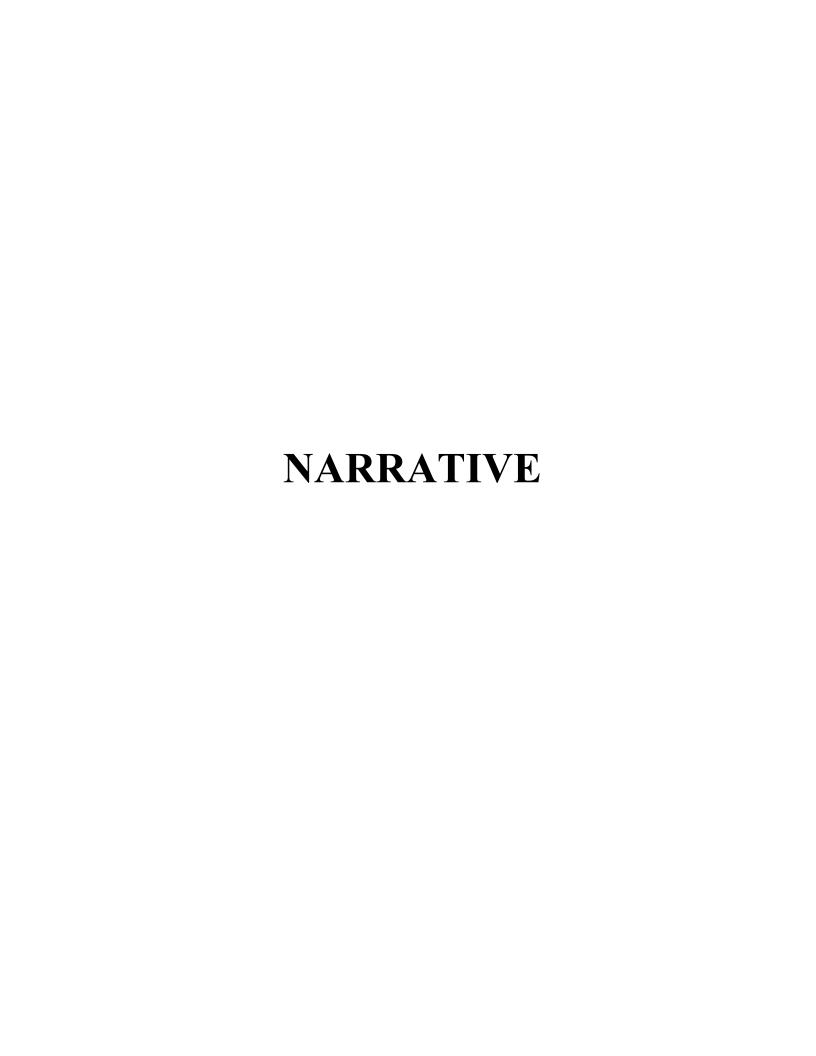
Phone: 202-524-5700

I, Matthew Senenman, P.E., hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 43200, Expiration Date: 12/20/24.

Rev March 19, 2024

Table of Contents

Narrative	3-6
Required ESDv Volume	7-8
Microbioretention Design Calculations	9-12
ESD Summary Chart	13
Soils Map	14



Narrative

The project is the new development of 23178 Three Notch Road, located on the northwest side of the intersection of Three Notch Road and Patuxent Beach Road. The total site area is 4.44 acres, although the disturbance area will only be 2.25 acres of the site. The limit of disturbance is comprised of 0.35 acre of existing impervious cover and 1.89 acre of existing pervious cover.

The site generally slopes from northwest to southeast, where there is an existing wetland The proposed site maintains similar drainage patterns, while treating the vehicular area with micro-bioretention facilities.

All soils onsite are hydrologic soil group "C' soils as mapped by the U.S. Department of Agriculture, Soil Conservation Service. There are no known streams and the site is not located within a 100-year floodplain. There is a wetland identified on the plans that is not to be disturbed other than minor improvements encroaching the 25' buffer. All existing steep slopes are hatched on the existing conditions plan and are impacted as little as possible. The site encompasses a driveway entrance to existing buildings and wooded area. The existing buildings and site features will be demolished within the extents of the limits of disturbance. The proposed development will be a car wash facility and parking lot, along with associated utilities and landscaping.

Because the limit of disturbance is comprised of more than 16.1% impervious cover, this is a new development The southern portion of the site is located in the St. Mary's River watershed and the northern part of the site is located in the Patuxent River watershed. Stormwater management requirements include providing water quality treatment for the required ESD volume, and water quantity treatment for the increase in runoff in the 10-year and 100-year storm.

Stormwater - Proposed Conditions - Evaluation of ESD Requirements

The required ESD volume is 4,540 cubic feet based on 1.26 acres of proposed impervious cover and a required impervious area requiring treatment (IART) of 0.91 acre and a Pe of 1.80" for new development. Quality treatment will be provided by micro-bioretention planters.

The surface micro-bioretention planters located within the parking lot to the east of the building will treat all of the New Development area (0.91 acre). This practice was chosen because the majority of the site will be covered by the proposed building and parking areas. Sheet flow within the parking lot will be routed to these facilities through curb cuts. Rip rap will be installed at these curb cuts for pre-treatment.

See Table 1 for a summary of the water quality treatment provided:

Table 1. ESD Facility Summary

Drainage Area #	SWM Facility #	Drainage Area (Acres)	To Treatment	Target ESDv	Provided ESDv	Target Volume using Structural Practices	Provided Volume with Structural Practices
BMP-1	Micro-Bioretention A1	0.459	34%	1555 cf	2414 cf	0 cf	0 cf
BMP-2	Micro-Bioretention A2	0.459	34%	1555 cf	3159 cf	0 cf	0 cf
BMP-3	Micro-Bioretention A3	0.423	32%	1431 cf	3019 cf	0 cf	0 cf
	Σ	1.341	100%	4540 cf	8592 cf	0 cf	0 cf
Required Site Area ESD=			4,540 cf				
Provided Site Area ESD=			8,592 cf				
TOTAL PROVIDED P _E :							
$PE = (12 \times ESD_V) / (R_V \times A) =$			3.41 inches	2.6" MAX; DRAIN	AGE EXCEEDS		
TOTAL PROVIDED Q_E :	·	·					
$Q_E = P_E X R_V =$	= 2.6 inches x 0.95 =		1.44 inches				
	Maximum Allowable Volun	ne ($Pe = 2.6$ ")					

Stormwater - Proposed Conditions - Quantity Analysis

Structur	ral Practices	Design	Volume									
Step 1: Det	ermine total	volume	treated by E	SD devices	8592	cfs						
Step 2: Cal	culate rainfal	l captui	red and treat	ed by ESD d	$evices = P_E$	= (12*ESD	(Rv*A) =	1.91	in			
	T 11 50				1 150			10/7		PP 11	•	
Step 3: Usi	_		MDE manua	I, determine	reduced RC	N, by inser	ting the deve	loped % Impe	rvious and th	ne PE achieve	ed	
	Imp % =	56% 70										
	$RCN_{RED} =$	/0										
Step 4: Cal	culate S and	Runoff	OE resulting	g from reduc	ed RCN							
P1=	1.8	IN										
$S_{RED} = (100)$	00/RCNred)-	10 =	4.29									
$Q_{RED} = (2.7)$	7-0.2*Sred)^2	2 / (2.7	$+0.8*S_{RED}$)	0.17	in							
Step 5: Cal	culate Volun	ne of Ru	inoff Result	ng from redu	iced RCN							
$V_{RED} = (Q_F)$	EED*A)/12 =	1386	cf									
0. (0.1	1 . 37 1	C.D.	00 1.1		1 1 .	D CD I						
Step 6: Cal	culate Volun	ne of Ru	ınoff resultıı	ng from pre-c	levelopment	RCN						
Spac = (100	00/RCN _{PRE})-1	0 -		4.29								
	7-0.2*Spre)^2		+ 0 8*Sppr) =		in							
	$_{RE}*A)/12 =$	7 (2.7	O.O SPRE)	1386								
v PRE - (QP	KE A)/12 -			1300	CI							
Sten 7: Cal	culate addition	nnal ma	nagement re	anired in str	l uctural RMI	D _C						
Stop /. Cal	Carato additit	,,,u, ,,,u	imgement re	quirou in sur	accarar DIVII	. 5						
$V_{STR} = V_{RE}$	D - VPRE=	0	cf									
JIK TAL												
Volume Pro	ovided by Un	dergrou	ind Stormted	h Facility								
V _{STECH} =		cf										

Sediment Control Design

Sediment control will be provided with a combination of silt fence, silt fence on pavement and inlet protection. Silt fence and silt fence on pavement will wrap the construction areas.

REQUIRED ESDv VOLUME

Project Name: Tidal Wave
Project Number: MDB230016.00
ESD COMPUTATIONS

Step 1 - Determine ESD Requirements

DATE: 1/29/2024 BY: SL CK: MCS

 THE PARTY OF THE P		
INITIAL SITE ASSESSMENT (SITE DATA):		4.44
SITE AREA		4.44 Acres
LIMIT OF DISTURBANCE		2.25 Acres
EXISTING IMPERVIOUS PERCENTAGE		0.35 = 15.5% Impervious
PROPOSED IMPERVIOUS PERCENTAGE		1.26 = 55.9% Impervious
DEVELOPMENT CATEGORY		New Development
100% IMPERVIOUS AREA TO BE TREATED		0.35 Acres
NEW IMPERVIOUS AREA TO BE TREATED		0.91 Acres
TOTAL IMPERVIOUS AREA TO BE TREATED		1.26 Acres
DRAINAGE AREA WITHIN HYDROLOGIC GROUP "A"		0.00 Acres = 0.0% of LOD
DRAINAGE AREA WITHIN HYDROLOGIC GROUP "B"		0.00 Acres = 0.0% of LOD
DRAINAGE AREA WITHIN HYDROLOGIC GROUP "C"		2.25 Acres = 100.0% of LOD
DRAINAGE AREA WITHIN HYDROLOGIC GROUP "D"		0.00 Acres = 0.0% of LOD
RCN (ASSUMING WOODS GOOD CONDITION)		RCN = 70
NEW DEVELOPMENT REQUIREMENTS		
A = 100% OF EXISTING IMPERVIOUS =	1.26 Acres	
$R_V = 0.05 + 0.009$ (% IMPERVIOUS) =	0.55	
P _E FOR EXISTING IMPERVIOUS =	1.80 inches	
REQUIRED $Q_E = P_E x R_V =$	1.00 inches	
REQUIRED ESD _V = $(P_E \times R_V \times A) / 12 =$	4540 cf	
TOTAL REQUIRED ESD _v :		
TOTAL REQUIRED ESDY.		
$P_E =$		1.80 inches
$ESD_V = ESD_V (Redev) =$		4540 cf = 0.104 ac-ft

Sub-Drainage Information

Drainage Area #	SWM Facility #	Drainage Area (Acres)	To Treatment	Target ESDv	Provided ESDv	Target Volume using Structural Practices	Provided Volume with Structural Practices
BMP-1	Micro-Bioretention A1	0.459	34%	1555 cf	2414 cf	0 cf	0 cf
BMP-2	Micro-Bioretention A2	0.459	34%	1555 cf	3159 cf	0 cf	0 cf
BMP-3	Micro-Bioretention A3	0.423	32%	1431 cf	3159 cf	0 cf	0 cf
	2	1.341	100%	4540 cf	8732 cf	0 cf	0 cf

Before Structural Practices		_
Required Site Area ESD=	4,540 cf	
Provided Site Area ESD=	8,732 cf	
TOTAL PROVIDED P _E :		
$PE = (12 \text{ X ESD}_V) / (R_V \text{ X A}) =$	3.46 inches	2.6" MAX; DRAINAGE EXCEEDS
TOTAL PROVIDED Q _E :		1
$O_v = P_v \times R_v = = 2.6 \text{ inches } \times 0.95 =$	1.44 inches	

Maximum Allowable Volume (Pe = 2.6")

MICRO-BIORETENTION DESIGN CALCULATIONS

Step 2 - ESD Design

PROP. BMP A1

Use of Micro-Bioretention (M-6)

0.459 20000 Drainage Area (acres) SF Proposed Impervious Area 0.459 20000 SF Percent Impervious 100.0% $\mathbf{R}\mathbf{v}$ 0.950 Target ESDv = 1438 CF

1) WQ Foundation Micro Bio-retention Facility Characteristics $Df = Depth \ of \ filter \ bed \ and \ sand \ layer$

= 2.00 FTk = Coefficient of permeability
Dp = Ponding depth
Tf = Design filter bed drain time = 0.40 FT/DAY = 1.000 FT = 3 DAYS

BIO-RETENTION DIMENSIONS

Af = Surface area of filter bed =480 SF

2) Required Surface Area for Filter Bed

Af (MIN) = 2% of Total Drainage Area (A) =400 SF

> 480 SF 400 SF

3) ESDv Provided

Volume Above Filter Bed

= 480 SFAf = Surface area of filter bed Aw = Area at water surface elevation = 1550 SF Dp = Ponding depth = 2.00 FT = 2030 CF Volume = [(Aw+Af)/2]*Dp

Volume in Filter Media

Af = Surface area of filter bed Df = Depth of filter bed = 480 SF = 2.00 FTVolume = Af*Df*0.4= 384 CF

Volume Provided

2030 CF 384 CF = 2414 CF

Maximum Allowable Volume (Pe = 2.6")

ESDv = (Pe*Rv*A)/12 == 4117 CF USE CALCULATED VOLUME

TOTAL $\ensuremath{\mathsf{ESD_{V}}}$ PROVIDED:

= 2414 CF TOTAL PE PROVIDED: 1.52 inches

TOTAL PERFORATED PVC UNDERDRAIN REQUIRED = 5% OF Af: = 24.0 FTTOTAL PERFORATED PVC UNDERDRAIN PROVIDED: = 63.0 FT

Step 2 - ESD Design

PROP. BMP A2

Use of Micro-Bioretention (M-6)

0.459 20000 Drainage Area (acres) SF Proposed Impervious Area 0.352 15313 SF Percent Impervious 76.6% $\mathbf{R}\mathbf{v}$ 0.739 Target ESDv = 1438 CF

1) WQ Foundation Planter Facility Characteristics $Df = Depth \ of \ filter \ bed \ and \ sand \ layer$

= 2.00 FTk = Coefficient of permeability = 0.40 FT/DAYDp = Ponding depth
Tf = Design filter bed drain time = 1.000 FT= 3 DAYS

BIO-RETENTION DIMENSIONS

Af = Surface area of filter bed = 1066 SF

2) Required Surface Area for Filter Bed

Af (MIN) = 2% of Total Drainage Area (A) = 400 SF

> 1066 SF 400 SF

3) ESDv Provided

Volume Above Filter Bed

= 1066 SFAf = Surface area of filter bed Aw = Area at water surface elevation = 2009 SF Dp = Ponding depth = 1.500 FT Volume = [(Aw+Af)/2]*Dp= 2306 CF

Volume in Filter Media

Af = Surface area of filter bed Df = Depth of filter bed Volume = Aw*Df*0.4 = 1066 SF = 2.00 FT= 853 CF

Volume Provided

2306 CF 853 CF = 3159 CF

Maximum Allowable Volume (Pe = 2.6")

ESDv = (Pe*Rv*A)/12== 3202 CFUSE CALCULATED VOLUME

TOTAL $\ensuremath{\mathsf{ESD_{V}}}$ PROVIDED:

= 3159 CF TOTAL PE PROVIDED: 2.56 inches

TOTAL PERFORATED PVC UNDERDRAIN REQUIRED = 5% OF Af: = 53.3 FT TOTAL PERFORATED PVC UNDERDRAIN PROVIDED: = 47.0 FT

Step 2 - ESD Design

PROP. BMP A3

Use of Micro-Bioretention (M-6)

0.423 18407 Drainage Area (acres) SF Proposed Impervious Area 0.332 14465 SF Percent Impervious 78.6% $\mathbf{R}\mathbf{v}$ 0.757 Target ESDv = 1438 CF

1) WQ Foundation Planter Facility Characteristics $Df = Depth \ of \ filter \ bed \ and \ sand \ layer$

= 2.00 FTk = Coefficient of permeability = 0.40 FT/DAYDp = Ponding depth
Tf = Design filter bed drain time = 1.000 FT= 3 DAYS

BIO-RETENTION DIMENSIONS

Af = Surface area of filter bed = 1076 SF

2) Required Surface Area for Filter Bed

Af (MIN) = 2% of Total Drainage Area (A) = 368 SF

> 1076 SF 368 SF

3) ESDv Provided

Volume Above Filter Bed

= 1076 SFAf = Surface area of filter bed Aw = Area at water surface elevation = 2010 SF Dp = Ponding depth = 1.000 FT = 1543 CF Volume = [(Aw+Af)/2]*Dp

Volume in Filter Media

Af = Surface area of filter bed Df = Depth of filter bed Volume = Aw*Df*0.4 = 1076 SF= 2.00 FT= 861 CF

Volume Provided

1543 CF 861 CF = 2404 CF

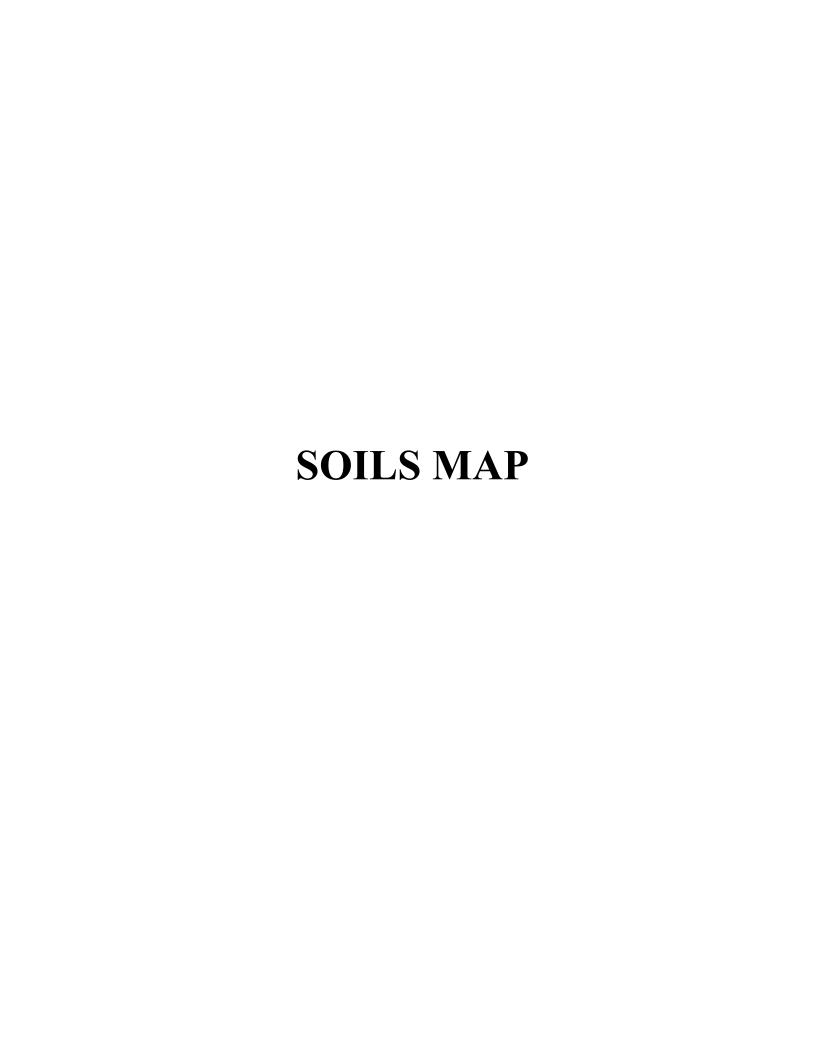
Maximum Allowable Volume (Pe = 2.6")

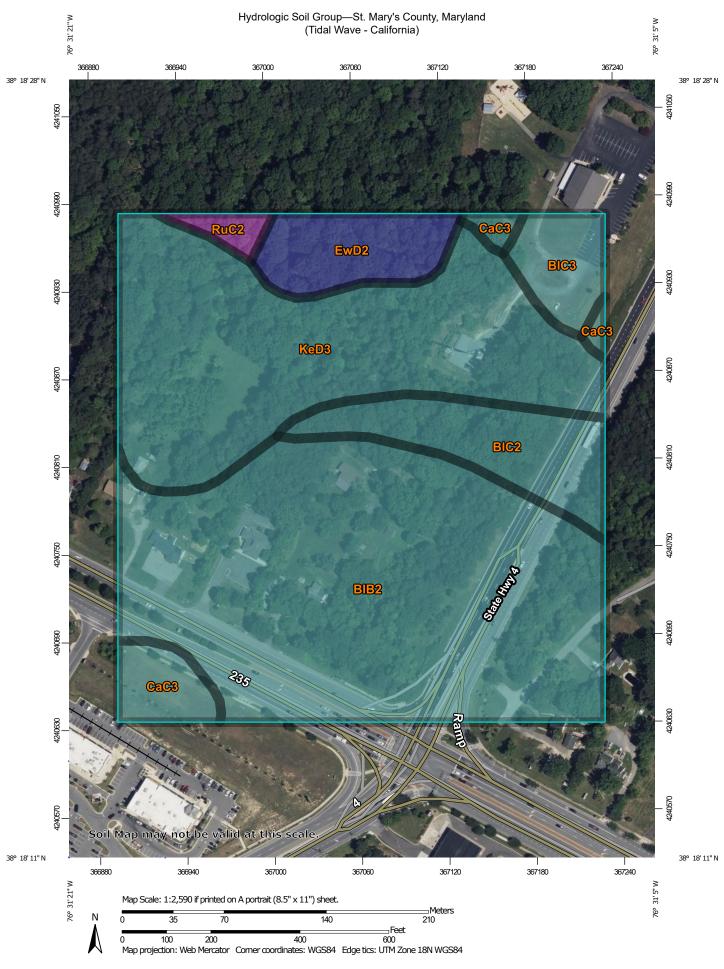
ESDv = (Pe*Rv*A)/12== 3019 CF USE CALCULATED VOLUME

TOTAL $\ensuremath{\mathsf{ESD_{V}}}$ PROVIDED:

= 2404 CF TOTAL PE PROVIDED: 2.07 inches

TOTAL PERFORATED PVC UNDERDRAIN REQUIRED = 5% OF Af: = 53.8 FT TOTAL PERFORATED PVC UNDERDRAIN PROVIDED: = 46.0 FT

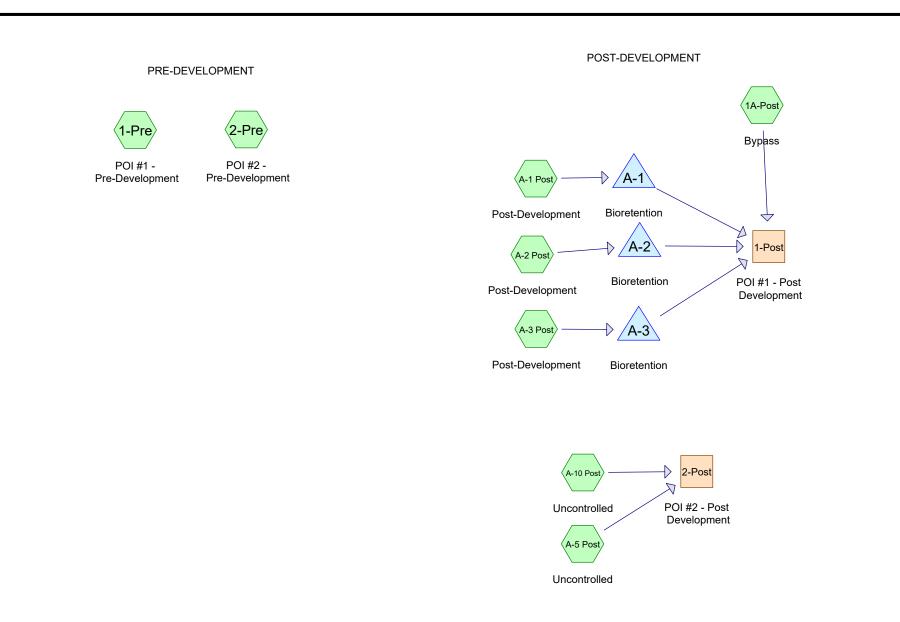




MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:20.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed В Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: St. Mary's County, Maryland Survey Area Data: Version 19, Sep 14, 2022 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. D Not rated or not available Date(s) aerial images were photographed: May 29, 2022—May 31. 2022 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
BIB2	Beltsville silt loam, 2 to 5 percent slopes, moderately eroded	С	13.5	46.8%
BIC2	Beltsville silt loam, 5 to 10 percent slopes moderately eroded	С	2.4	8.2%
BIC3	Beltsville silt loam, 5 to 10 percent slopes, severely eroded	С	1.1	3.7%
CaC3	Caroline silt loam, 5 to 10 percent slopes, severely eroded	С	1.1	3.7%
EwD2	Evesboro-Westphalia complex, 12 to 20 percent slopes, moderately eroded	В	1.6	5.6%
KeD3	Kempsville fine sandy loam, 10 to 15 percent slopes, severely eroded	С	8.9	31.0%
RuC2	Rumford loamy sand, 5 to 10 percent slopes, moderately eroded	A	0.3	1.1%
Totals for Area of Inter	rest		28.9	100.0%











Routing Diagram for Tidal Wave St. Mary's County - MDB230016.0

Prepared by Bohler Engineers, Printed 3/19/2024

HydroCAD® 10.20-4a s/n 03478 © 2023 HydroCAD Software Solutions LLC

Tidal Wave St. Mary's County - MDB230016.00 Prepared by Bohler Engineers HydroCAD® 10.20-4a s/n 03478 © 2023 HydroCAD Software Solutions LLC

Printed 3/19/2024

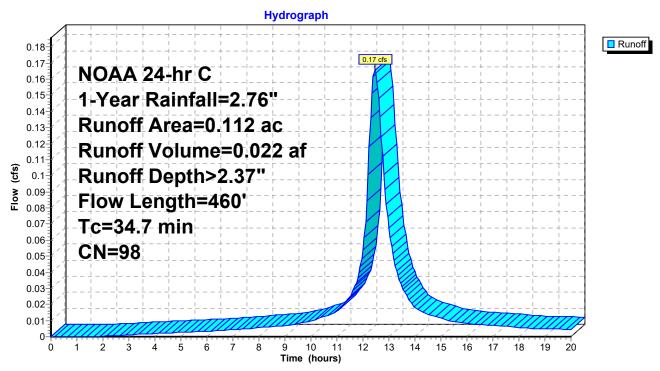
Page 1

Rainfall Events Listing

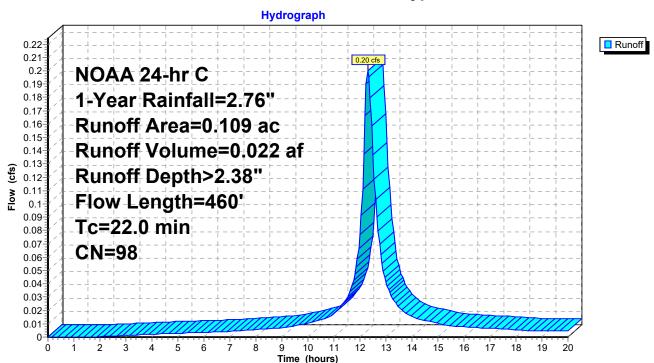
Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-Year	NOAA 24-hr	С	Default	24.00	1	2.76	2
2	10-Year	NOAA 24-hr	С	Default	24.00	1	5.24	2
3	100-Year	NOAA 24-hr	С	Default	24.00	1	9.02	2

Page 2

Subcatchment 1-Pre: POI #1 - Pre-Development

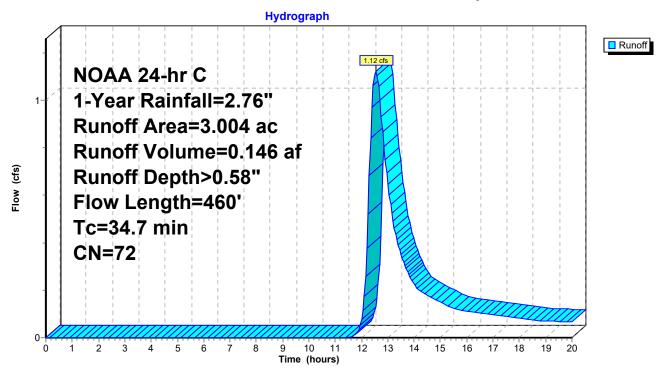


Subcatchment 1A-Post: Bypass

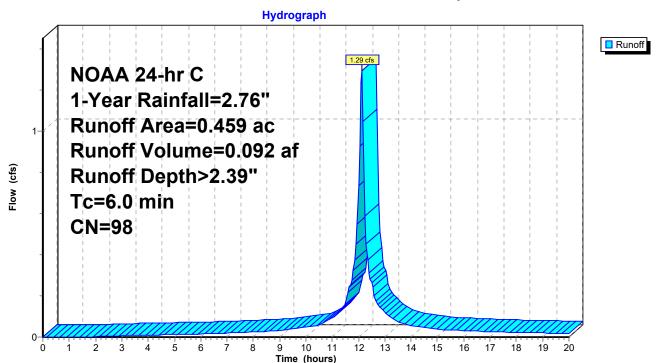


Page 3

Subcatchment 2-Pre: POI #2 - Pre-Development

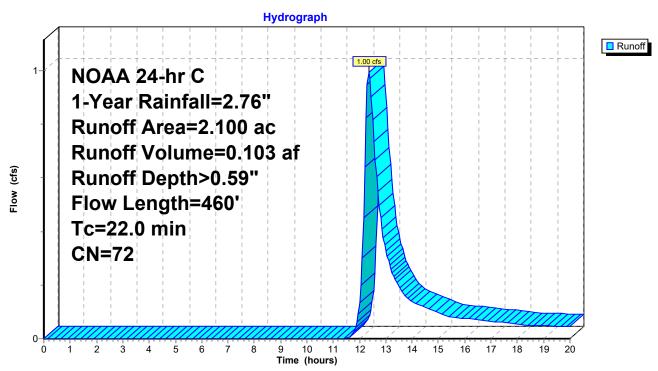


Subcatchment A-1 Post: Post-Development

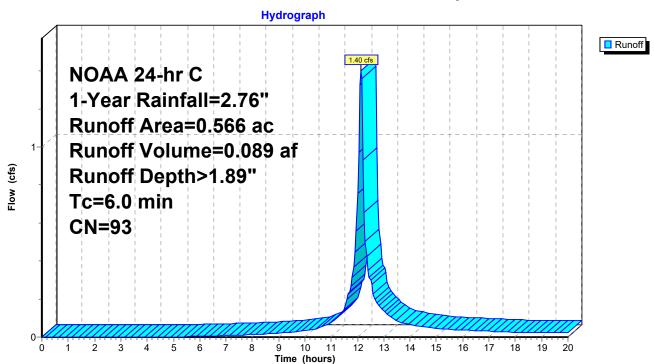


Page 4

Subcatchment A-10 Post: Uncontrolled

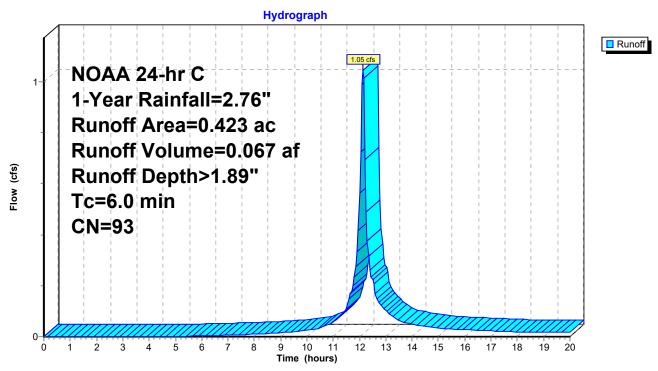


Subcatchment A-2 Post: Post-Development

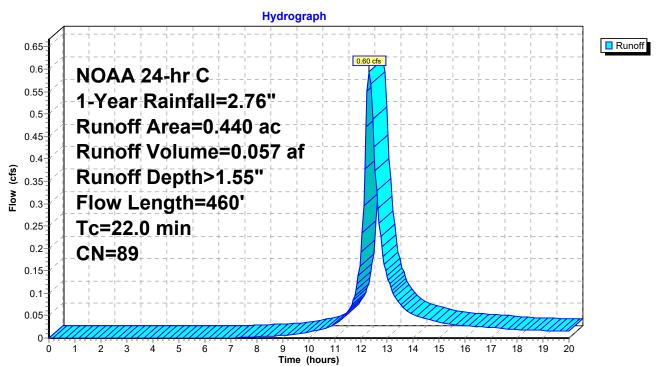


Page 5

Subcatchment A-3 Post: Post-Development

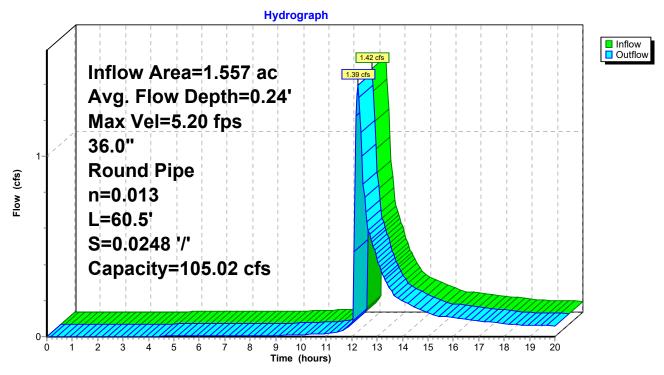


Subcatchment A-5 Post: Uncontrolled

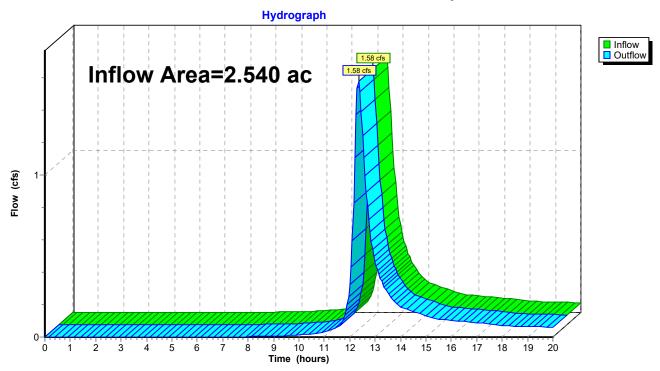


Page 6

Reach 1-Post: POI #1 - Post Development

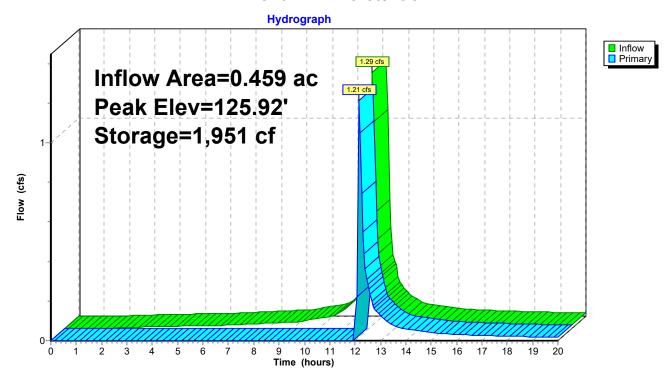


Reach 2-Post: POI #2 - Post Development

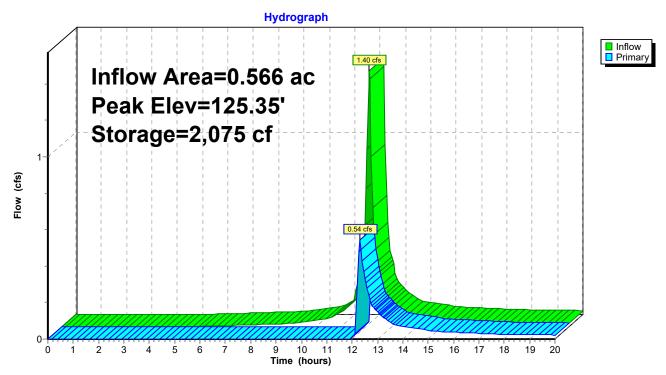


Page 7

Pond A-1: Bioretention



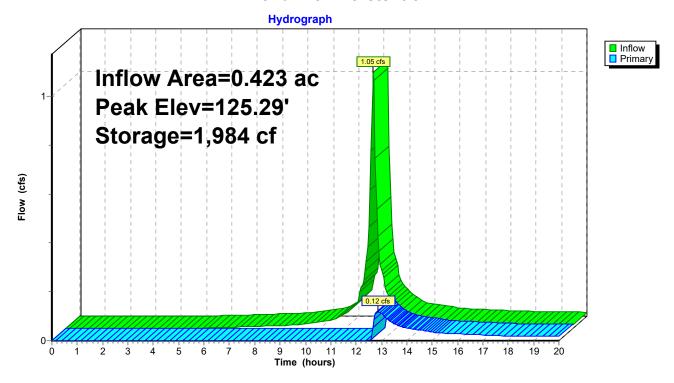
Pond A-2: Bioretention



Prepared by Bohler Engineers
HydroCAD® 10.20-4a s/n 03478 © 2023 HydroCAD Software Solutions LLC

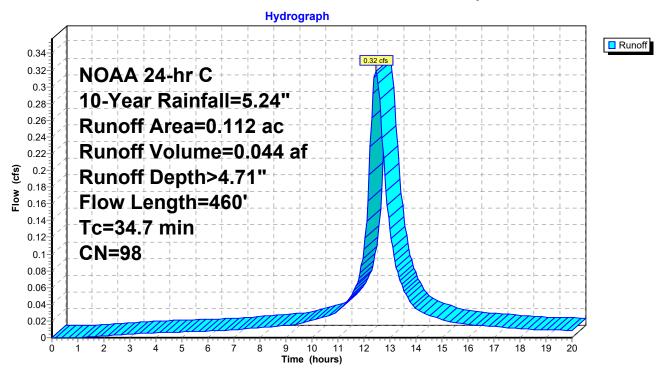
Page 8

Pond A-3: Bioretention

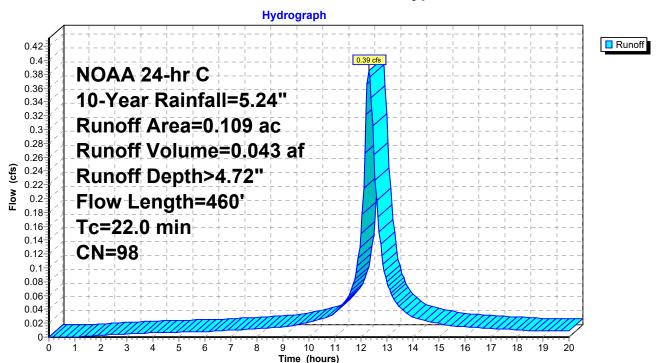


Page 9

Subcatchment 1-Pre: POI #1 - Pre-Development

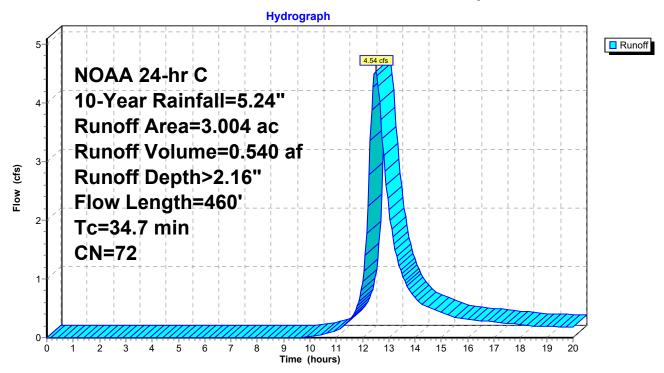


Subcatchment 1A-Post: Bypass

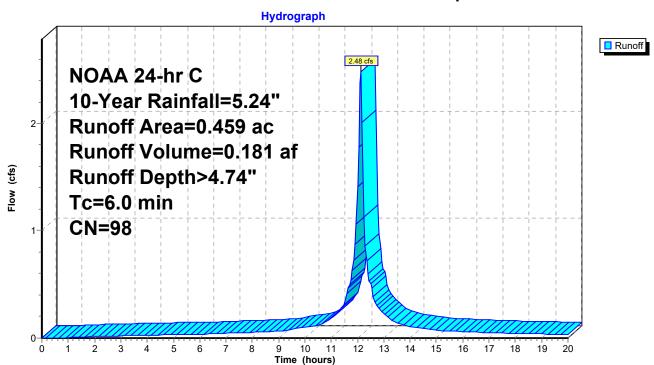


Page 10

Subcatchment 2-Pre: POI #2 - Pre-Development

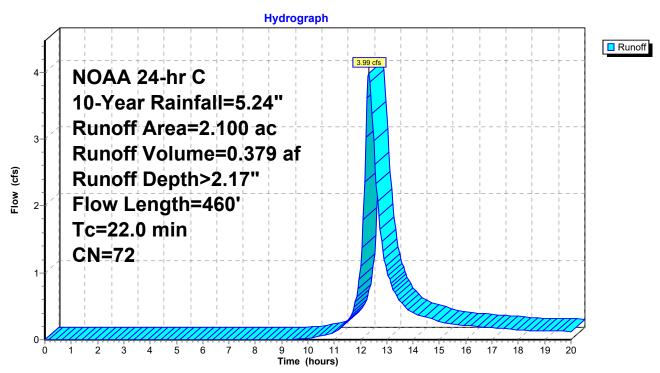


Subcatchment A-1 Post: Post-Development

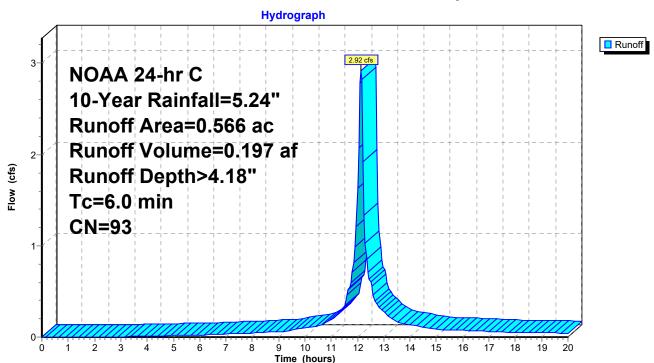


Page 11

Subcatchment A-10 Post: Uncontrolled

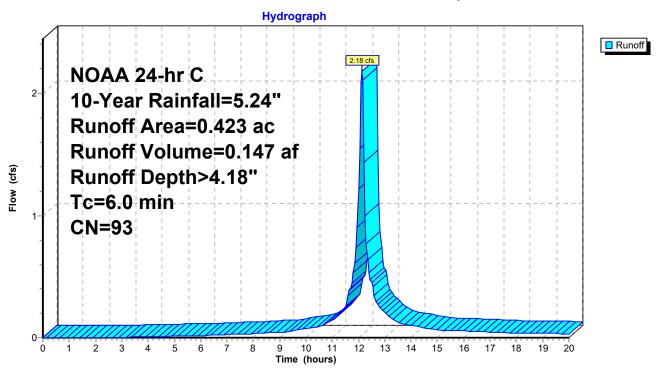


Subcatchment A-2 Post: Post-Development

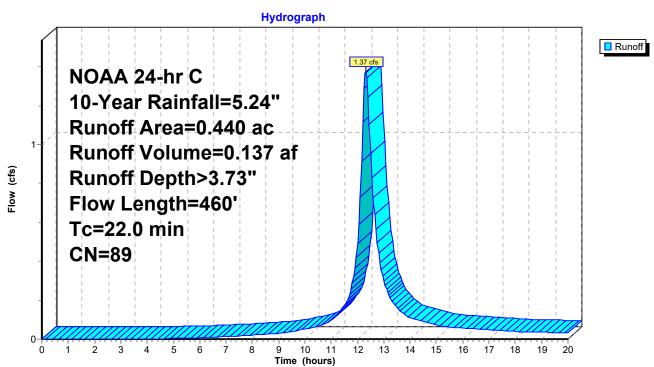


Page 12

Subcatchment A-3 Post: Post-Development

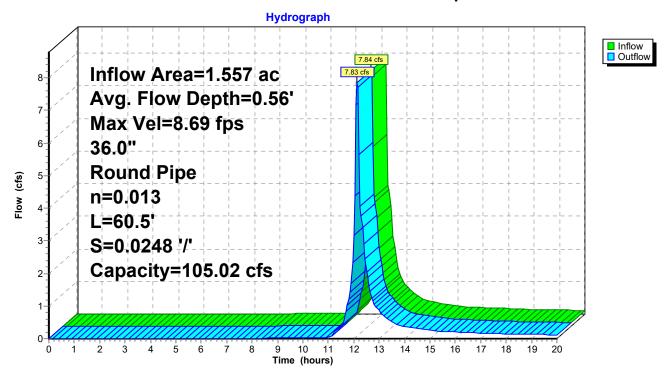


Subcatchment A-5 Post: Uncontrolled

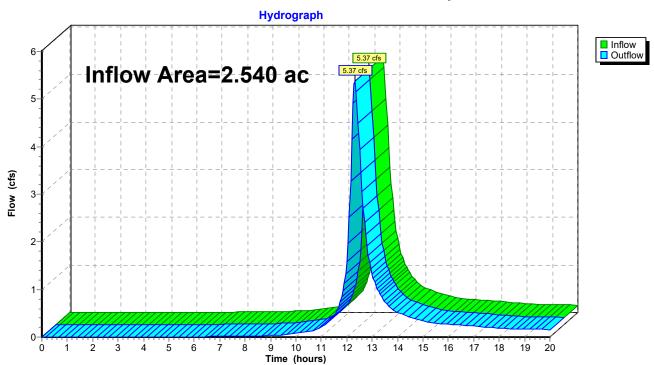


Page 13

Reach 1-Post: POI #1 - Post Development

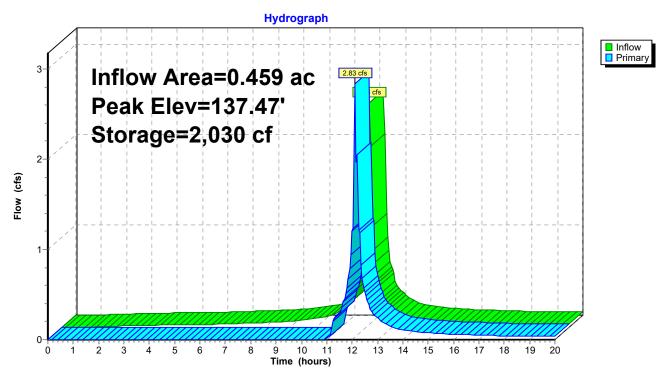


Reach 2-Post: POI #2 - Post Development

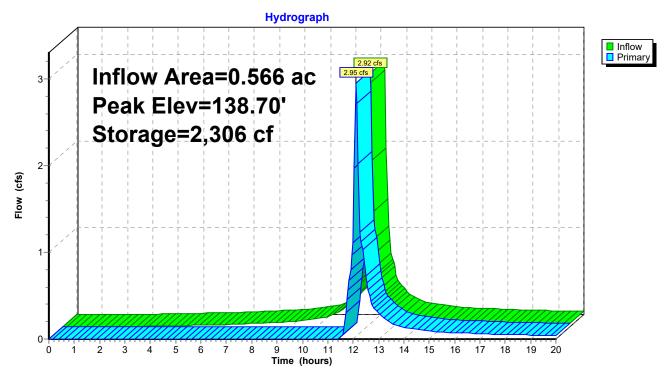


Page 14

Pond A-1: Bioretention

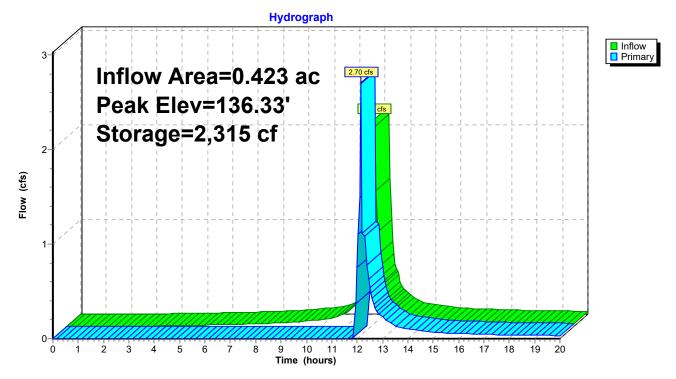


Pond A-2: Bioretention



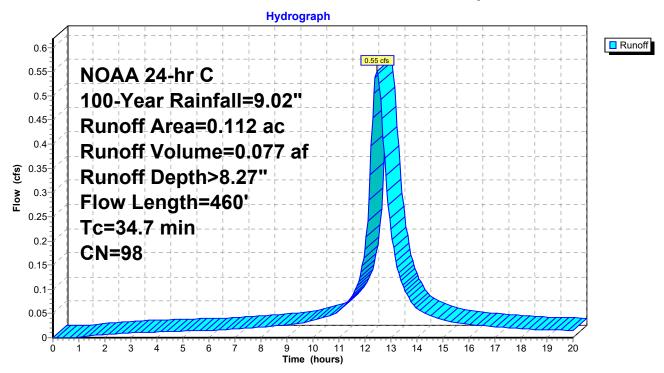
Page 15

Pond A-3: Bioretention

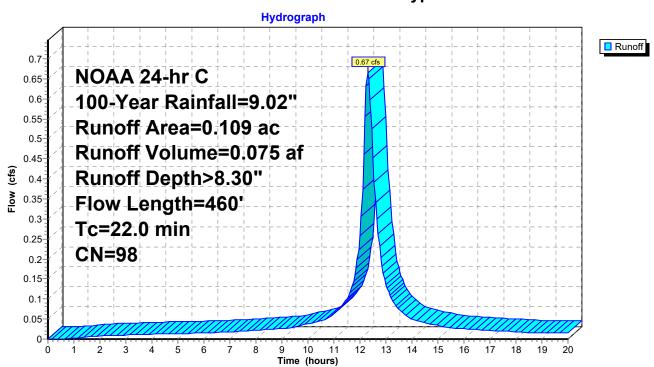


Page 16

Subcatchment 1-Pre: POI #1 - Pre-Development

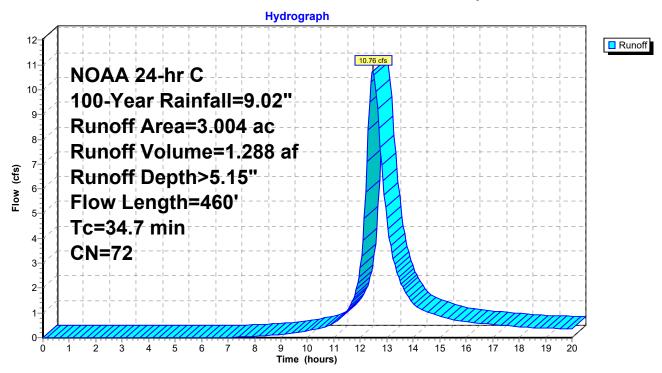


Subcatchment 1A-Post: Bypass

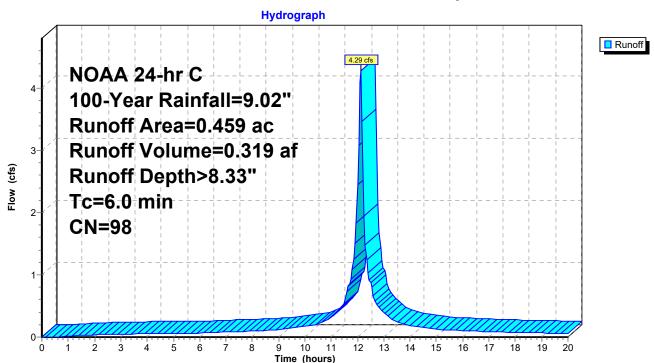


Page 17

Subcatchment 2-Pre: POI #2 - Pre-Development

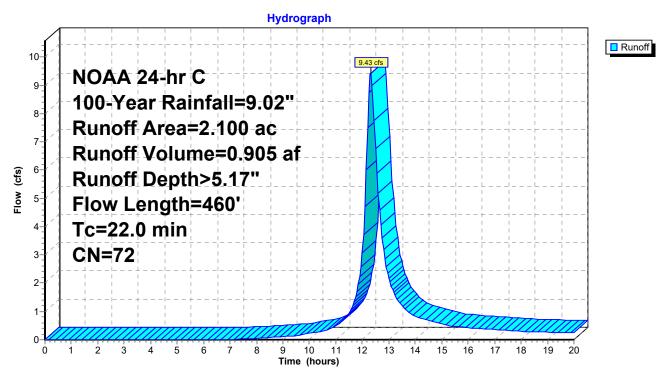


Subcatchment A-1 Post: Post-Development

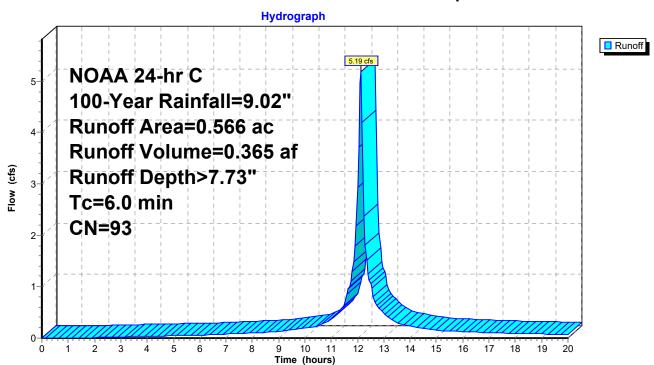


Page 18

Subcatchment A-10 Post: Uncontrolled

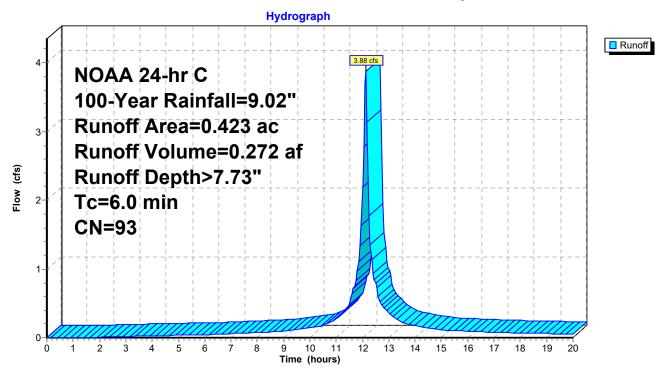


Subcatchment A-2 Post: Post-Development

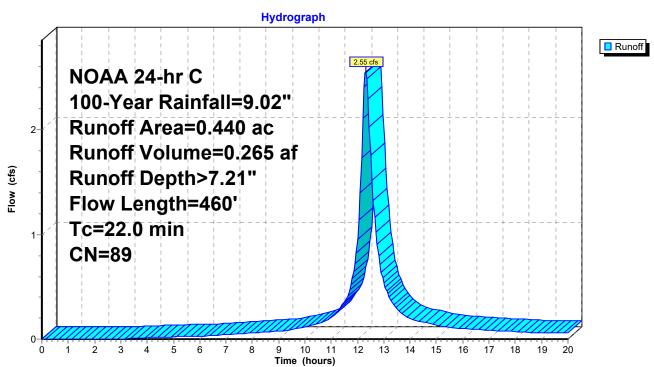


Page 19

Subcatchment A-3 Post: Post-Development

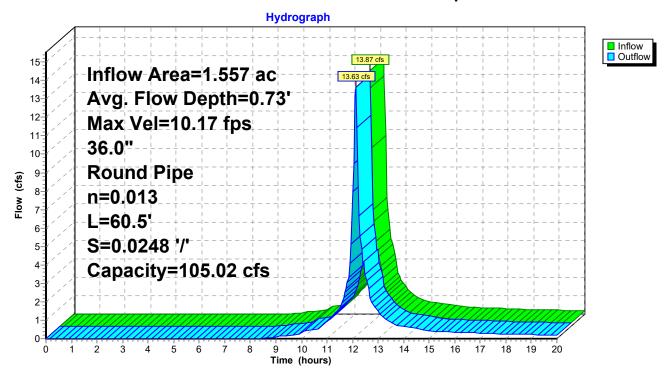


Subcatchment A-5 Post: Uncontrolled

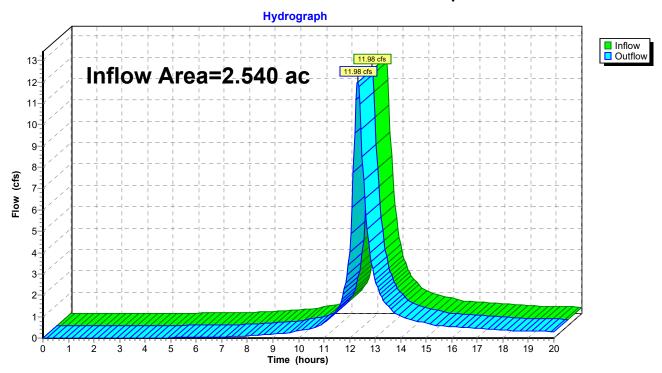


Page 20

Reach 1-Post: POI #1 - Post Development

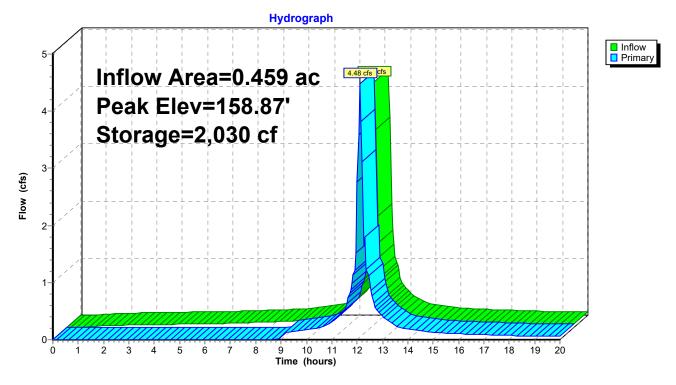


Reach 2-Post: POI #2 - Post Development

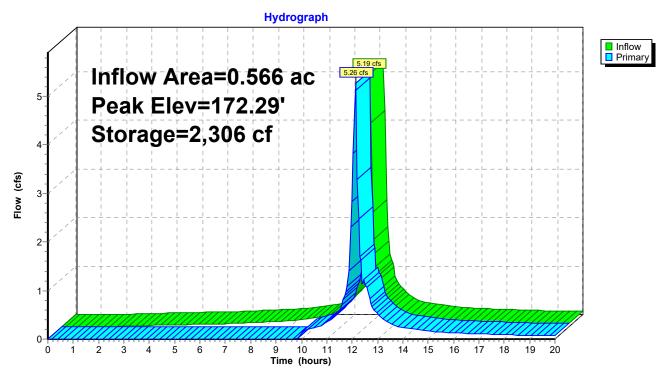


Page 21

Pond A-1: Bioretention



Pond A-2: Bioretention



Page 22

Pond A-3: Bioretention

